BUILDER/CONTRACTOR RESPONSIBILITIES

<u>Drawing Validity</u> — These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification

Builder Acceptance of Drawings - Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice Sept 86 Section 4.2.1) (Mar 05 Section 4.4.1)

Code Official Approval - It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance - The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable

Building Erection — The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Proctice Sept 86 Section 7.9.1) (Mar 05 Section 7.10.3)

Discrepancies — Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal All bolt joints with A325 Type 1 bolts are specified as snug—tightened joints, unless noted otherwise, in Building plans will govern. (AISC Code of Standard Practice Sept 86 Section 3.3) (Mar 05 Section 3.3)

Materials by Others — All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers

Correction of Errors — Normal erection operations include the correction of minor misfits by moderate amounts of reaming, chipping, welding or cutting and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in the member configuration should be reported immediately to the owner and fabricator by the erector, to enable whoever is responsible either to correct the error or to approve the most efficient and economical method of correction to be used by others. (AISC Code of Standard Practice Sept 86 Section 7.12)(Mar 05 Section 7.14)

Modification of the Metal Building from Plans — The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

<u>Safety Commitment</u> — The Metal Building Manufacturer has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of the building manufacturer. It is strongly recommended that safe working conditions and accident prevention is the top priority of any job site. Local, State and Federal safety and health standards, whether standard statutory or customary, should always be followed to help ensure worker safety. Make certain all employees know the safest and most productive way to erect a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended. For purposes of determining lift requirements, no bundles supplied by the manufacturer will exceed 4000 lbs. For further information also reference the bill of materials for individual member weights of other structural members. If additional information is required contact the customer service

Foundation Design — The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2

Dissimilar Materials - Never allow your roof to come in contact with, or water runoff from, any dissimilar metal including but not limited to: Copper and Arsenic Salts used in treated lumber, Calcium used in concrete, mortar

Debris Removal — Any foreign debris such as sawdust, dirt, animal droppings, etc. will cause corrosian of the roof, gutters, trim, etc. if left on building surfaces for a long enough time. The roof should be periodically inspected for such conditions and if found, they should be removed.

Rev. 10/18/12

Shop Primed Steel - All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by a corrosion resistant coating are pointed with one coat of shop primer meeting the performance requirements of SSPC Paint Specification No. 15. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum, the hand tool cleaning method SSPC-SP2 (Steel Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop Primed steel stored in the field pending erection should be kept free of the ground and so positioned as to minimize water—holding pockets, dust, mud and other contamination of the primer film. Repairs of damage to primed surfaces and/or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. The Manufacturer is not responsible for deterioration of the shop coat of primer or corrosion that may result from exposure to atmospheric and environmental conditions, nor the compatibility of the primer to any field applied coating. Minor abrasions to the shop coat (including galvanizing) caused by handling, loading, shipping unloading and erection after painting or galvanizing are unavoidable. Touch—up of these minor abrasions is the responsibility of the End Customer (MBMA 06 IV 4.2.4)

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1011 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, or other than flange braces, conform to ASTM 36 minimum. Hollow structural shaped conform to ASTM A500 grade b, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold form light gage steel members conform to the requirements of ASTM A1011 SS Grade 55 or ASTM A1011 HSLAS Class 1 Grade 55, with a minimum yield point of 55 ksi.

accordance with the "Specification for Structural Joints using ASTM A325 or A490 bolts, June 30, 2004". Pretensioning methods, including turn-of-nut and calibrated wrench are not required unless noted otherwise.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacture nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using monufocture's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly

X-bracing (if applicable) is to be installed to a taut condition with all slack removed. Do not tighten beyond

The design collateral load has been uniformly applied to the design of the building. Hanging loads are to be attached to the purlin web. This may not be appropriate for heavily concentrated loads. Any attached load in excess of 150 pounds shall be accounted for by special design performed by a licensed engineer using concentrated loads and may require separate support members within the roof system.

DESIGN LOADING

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY:

CBC 13 THE BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

1.00

FRAME / ROOF DEAD LOAD 2.000 PSF SUPERIMPOSED COLLATERAL (LIGHTS) 4.5 PSF FRAME / ROOF LIVE LOAD 12 /20.00 PSF RISK CATEGORY II - Normal GROUND SNOW LOAD (Pa) 20.000@SF SNOW LOAD IMPORTANCE FACTOR (Is) 1,0000 FLAT ROOF SNOW LOAD (Pr) 20 PSF SNOW EXPOSURE FACTOR (Ce) 1.0

WIND LOAD ULTIMATE WIND SPEED 110 MPH C WIND EXPOSURE CATEGORY TOPOGRAPHICAL FACTOR 1.0

THERMAL FACTOR (Ct)

INTERNAL PRESSURE COEFFICIENT (GCpi) 0.18 /-0.18 ZONE 4. COMPONENT WIND LOAD < 10FT2 29.826 PSF PRESSURE -32.354 PSF SUCTION

ZONE 5, COMPONENT WIND LOAD < 10FT2 29.826 PSF PRESSURE -39.861 PSF SUCTION

ZONES PER ASCE 7-10; FIG. 30.4-1 ZONES PRESSURES SHOWN ARE UN-FACTORED

RAIN INTENSITY 5-MINUTE DURATION, 5-YEAR RECURRENCE (I1) 2.5600 IN/HOUR

5-MINUTE DURATION, 25-YEAR 3.2200 IN/HOUR RECURRENCE (12)

SEISMIC LOAD SEISMIC IMPORTANCE FACTOR (Ie) 1.00 S_s 2.3707 S_{Ds} 1.0000 S1 0.9072 S_{D1} 0.9070 SITE CLASS ___D SEISMIC DESIGN CATEGORY Ε

ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

	TRANSVERSE	LONGIT FRONT	UDINAL BACK
BASIC FORCE RESISTING SYSTEM*	<u>C4</u>	B4	B4
RESPONSE MODIFICATION COEFFICIENT(R)	3.25	3.25	3.25
SYSTEM OVER-STRENGTH FACTOR(Ω_0)	3.0000	2.0000	2.0000
SEISMIC RESPONSE COEFFICIENT(Cs)	0.308	0.308	0.308
BLDG DESIGN BASE SHEAR (V)	_12.72_(k)	12.6	8_(k)
THE TOTAL COMPANY ASSESSMENT			

THE TRANSVERSE DIRECTION IS PARALLEL TO THE RIGID FRAMES THE LONGITUDINAL DIRECTION IS PERPENDICULAR TO THE RIGID FRAMES

> BASIC FORCE RESISTING SYSTEM* C4 STEEL ORDINARY MOMENT FRAME B4. STEEL ORDINARY CONCENTRIC BRACED FRAMES STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY

DETAILED FOR SEISMIC RESISTANCE G3. INVERTED PENDULUM SYSTEMS CANTILEVERED COLUMN SYSTEMS

	DRAW	/ING INDEX
ISSUE	PAGE	DESCRIPTION
Α	C1	COVER SHEET
0	F1	ANCHOR BOLT PLAN
0	F2	ANCHOR BOLT REACTIONS
0	F3	ANCHOR BOLT DETAILS
Α	E1	ROOF FRAMING PLAN
Α	E2	FRONT SIDEWALL
Α	E3	BACK SIDEWALL
Α	E4	LEFT ENDWALL
Α	E5	RIGHT ENDWALL
Α	E6	FRAME CROSS SECTION
Α	DET1-11	STANDARD DETAILS
-		

DRAWING STATUS

FOR APPROVAL

THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

X FOR CONSTRUCTION PERMIT THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR ERECTOR INSTALLATION" CAN BE CONSIDERED AS COMPLETE.

FOR ERECTOR INSTALLATION FINAL DRAWINGS FOR CONSTRUCTION

> FOR QUESTIONS OR ASSISTANCE 1-905-477-1894 MONDAY -- FRIDAY 7:30AM TO 5:00PM

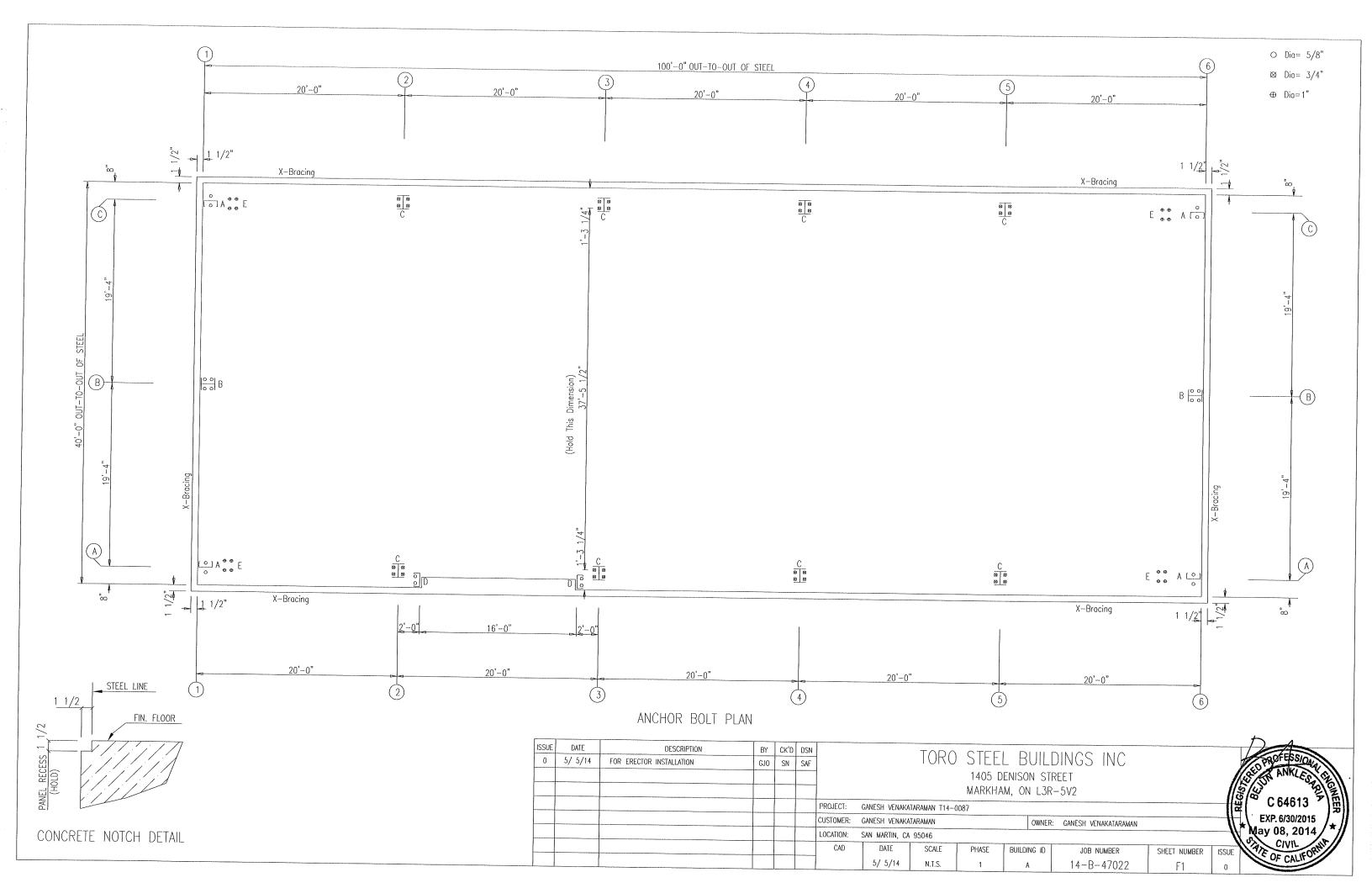
ENGINEERING SEAL

THIS CERTIFICATION COVERS PARTS MANUFACTURED AND DELIVERED BY THE MANUFACTURER ONLY,
AND EXCLUDES PARTS SUCH AS DOORS, WINDOWS,
FOUNDATION DESIGN AND ERECTION OF THE BUILDING.

THESE DRAWINGS AND THE METAL BUILDING SYSTEM THEY REPRESENT ARE THE PRODUCT OF AN AFFILIATE OF NCI GROUP, INC. - 10943 N. SAM HOUSTON PARKWAY W., HOUSTON, TX 77064. THE PROFESSIONAL ENGINEER WHOSE SEAL APPEARS HEREON IS EMPLOYED BY AN AFFILIATE OF NCI GROUP, INC. AND IS NOT THE ENGINEER-OF-RECORD FOR THE OVERALL PROJECT.

BUILDING SIZE: 40'-0" x 100'-0" x 25'-0" 3.0:12 ISSUE DATE DESCRIPTION BY CK'D DSN TORO STEEL BUILDINGS INC Α 5/ 5/14 FOR CONSTRUCTION PERMIT GJ0 SN SAF 1405 DENISON STREET MARKHAM, ON L3R-5V2 GANESH VENAKATARAMAN T14-0087 CUSTOMER: GANESH VENAKATARAMAN OWNER: GANESH VENAKATARAMAN LOCATION: SAN MARTIN, CA 95046 CAD DATE SCALE PHASE BUILDING ID JOB NUMBER SHEET NUMBER ISSUE 5/5/14 N.T.S. 14-B-47022 C1





GENERAL NOTES

1. THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.

2. REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENCINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.

3. THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

DESIGN
4. THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR THE THE METAL BUILDING MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE MACHOR BOLT DIAMETER ONLY TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IS NOT RESPONSIBLE FOR THE ANCHOR BOLT EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN AND EMBELORIZED THE OFFICE AND THE PROPERTY OF THE PROPER THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD ASSURE HIMSELF EMBELDMENTS, ITTLE LINU USE CUSTOMER SHOULD ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING SITE. THE BULLDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES, (SECTION A3 MBMA 2006 METAL BUILDING SYSTEMS MANUAL)

5. BOTTON OF ALL BASE PLATES ARE AT THE SAME ELEVATION.

(UNLESS NOTED)
6. ANCHOR RODS ARE ASTM F1554 GRADE 36 MATERIAL UNLESS NOTED OTHERWISE.

BUILDING BRACING REACTIONS

Loc	II Line	Col Line	± F Win	Reactions nd Vert	(k) - Seism Horz	ic — Vert	Panel_S (lb/ Wind	Shear (t) Seis
L_EW F_SW	1 A	B,A 1,2 5,6	3.1 2.9 2.9	4.6 3.3 3.3	1.5 3.2	2.3 3.7		
R_EW B_SW	6 C	A,B 6,5 2,1	3.1 2.9 2.9	3.3 3.3 3.3	3.2 1.5 3.2 3.2	3.7 2.3 3.7 3.7		

ISSUE

0

Frm	Col		_Bott		Plote (in)		Grout
Line	Line	Qty	Dia	Width	Length	Thick	(in)
1	С	2	0.625	7.000	12.00	0.250	0.0
1	В	4	0.625	6.000	12.00	0.375	0.0
1	Α	2	0.625	7.000	12.00	0.250	0.0
6	Α	2	0.625	7.000	12.00	0.250	0.0
6	В	4	0.625	6.000	12.00	0.375	0.0
6	C	2	0.625	7.000	12.00	0.250	0.0

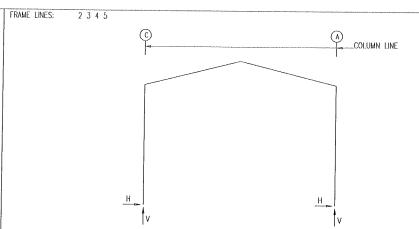
NOTES FOR REACTIONS

BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

> WIDTH (FT)
> LENGTH (FT)
> EAVE HEIGHT (FT)
> ROOF SLOPE (rise/12)
> DEAD LOAD (psf)
> COLLATERAL LOAD (psf)
> ROOF LIVE LOAD (psf)
> FRAME LIVE LOAD (psf)
> ROOF SNOW LOAD (psf)
> ROOF SNOW LOAD (psf) = 40 = 100 = 25 / 25 = 3.0:12 / 3.0:12 = 2.000 = 4.5 = 20.000 = 110 GROUND SNOW LOAD (psf) WIND SPEED (MPH) = 110 = CBC 13 = C WIND CODE CLOSED/OPEN = Closed = 1.00 IMPORTANCE - SEISMIC SEISMIC ZONE

ANCHOR	ROLT	SUMMARY

(Qly	Locate	Dia (in)	Туре	Proj (in)
	4 16 32 16	Jomb Endwoll Frome WindBent	5/8" 5/8" 3/4" 1"	F1554 F1554 F1554 F1554	2.00 2.00 2.50 3.00



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col	Anc	Bolt	Base_P	late (in)	Thick	Grout
Line	Line	Qty	Dia	Width	Length		(in)
2*	C	4	0.750	6.000	10.50	0.375	0.0
2*	A		0.750	6.000	10.50	0.375	0.0

Frame lines: 2 3 4 5

2 3 4 5

Frame lines:

RIGID FRAME: BASIC COLUMN REACTIONS (k) Frame Column -----Dead-----Collateral-Line Horiz Horiz Vert 4.4 -3.7 5.7 -11.2 0.2 -0.2 Frome Column --Wind_Left2- -Wind_Right2- --Wind_Long1- --Wind_Long2- -Seismic_Left Seismic_Right |
Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert |
2* C -7.2 -7.6 2.9 0.0 2.9 -10.8 2.1 -10.2 -1.6 -2.0 1.6 2.0 |
2* A -2.9 0.0 7.2 -7.6 -2.1 -10.2 -2.9 -10.8 -1.6 2.0 1.6 -2.0
 Frome
 Column
 -Seismic_Long
 F1UNB_SL_L F1UNB_SL_R

 Line
 Line
 Horiz
 Vert
 Horiz
 Vert
 Horiz
 Vert

 2*
 C
 0.0
 -4.8
 1.2
 7.4
 1.2
 4.3

 2*
 A
 0.0
 -4.8
 -1.2
 4.3
 -1.2
 7.4
 Line 2* 2*

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k) Col Line Press Horz 0.0 -6.5 0.0 Wind Left1 Wind_Right1 Wind_Left2 Wind_Right2 Line Vert 0.3 Vert Vert 1.6 Horz 0.0 3.1 Vert -2.1 -10.7 3.4 Horz 0.0 0.0 3.1 Vert -2.0 -0.7 -6.7 Horz 0.0 3.1 0.0 Vert -2.1 -10.7 3.4 Horz 0.0 0.0 3.1 Vert -2.0 -0.7 -6.7 0.3 0.4 Frm Col Suct Horz 0.0 7.2 Wind_Long1 Wind_Long2 Seis_Left Seis_Right E1UNB_SL_L-Horz Vert 0.0 1.8 E1UNB_SL_R-Line Line Horz 0.0 0.0 0.4 Horz 0.0 0.4 Horz 0.0 2.0 0.0 Horz 0.0 0.0 2.0 Vert -0.1 2.9 Vert Vert 0.1 Horz 0.0 0.0 0.0 0.0 0.0 Vert 0.3 3.8 1.8 -4.9 3.8 0.0 0.0 Col Dead Vert 0.3 Snow Vert 1.6 4.9 1.6 Wind_Right1 Wind_Right2 Horz Vert 0.0 3.4 Wind Left2 Press Vert 0.4 1.1 Vert 3.4 -10.7 Horz 3.1 0.0 Horz 0.0 3.1 Vert -6.7 -0.7 -2.0 Horz 0.0 3.1 0.0 Horz 0.0 -6.5 0.0 -6.7 -0.7 -2.0 1.1 0.3 -10.7 -2.1 0.4 0.0 Wind_Long1 Horz Vert Col Suct Seis_Left Seis_Right E2UNR SL L-E2UNB_SL_R-Horz 0.0 0.4 0.0 Vert -1.7 -3.6 -2.0 Horz 0.4 0.0 0.0 Horz 0.0 2.0 0.0 Horz 2.0 0.0 0.0 Vert -2.8 2.9 -0.1 Vert 3.3 -3.4 0.1 Horz 0.0 0.0 0.0 Horz 0.0 0.0 0.0 Vert 0.3 3.8 1.8 0.0 -1.3

JOB NUMBER

14-B-47022

SHEET NUMBER

F2

ISSUE

C 64613 EXP. 6/30/2015

May 08, 2014

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				T	T	
-	DATE	DESCRIPTION	BY	CK'D	DSN	TODO CTEEL DIMIDINGS INO
	5/ 5/14	FOR ERECTOR INSTALLATION	GJO	SN	SAF	TORO STEEL BUILDINGS INC
						1405 DENISON STREET
						MARKHAM, ON L3R-5V2
						PROJECT: GANESH VENAKATARAMAN T14-0087
4						PROJECT: GANESH VENAKATARAMAN T14-0087
4						CUSTOMER: GANESH VENAKATARAMAN OWNER: GANESH VENAKATARAMAN

LOCATION:

SAN MARTIN, CA 95046

SCALE

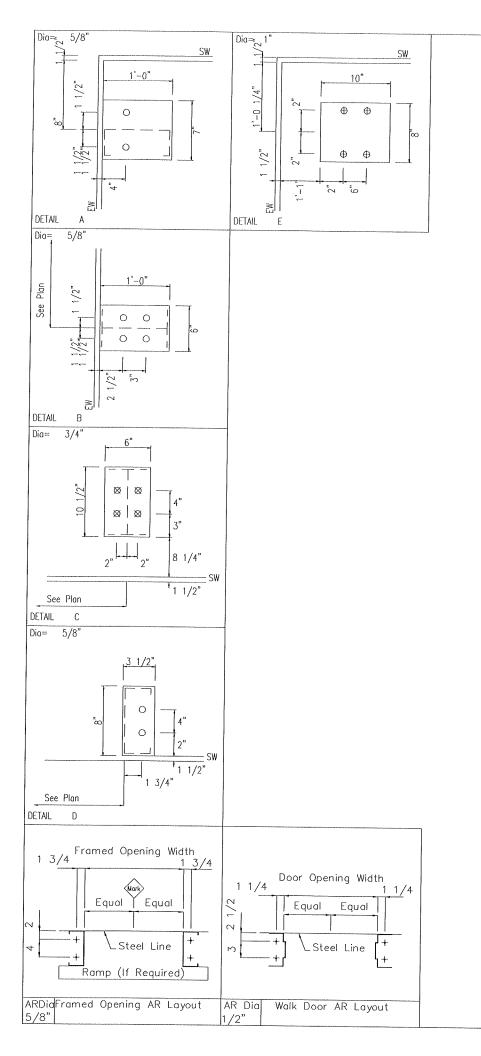
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PHASE

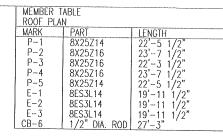
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DATE

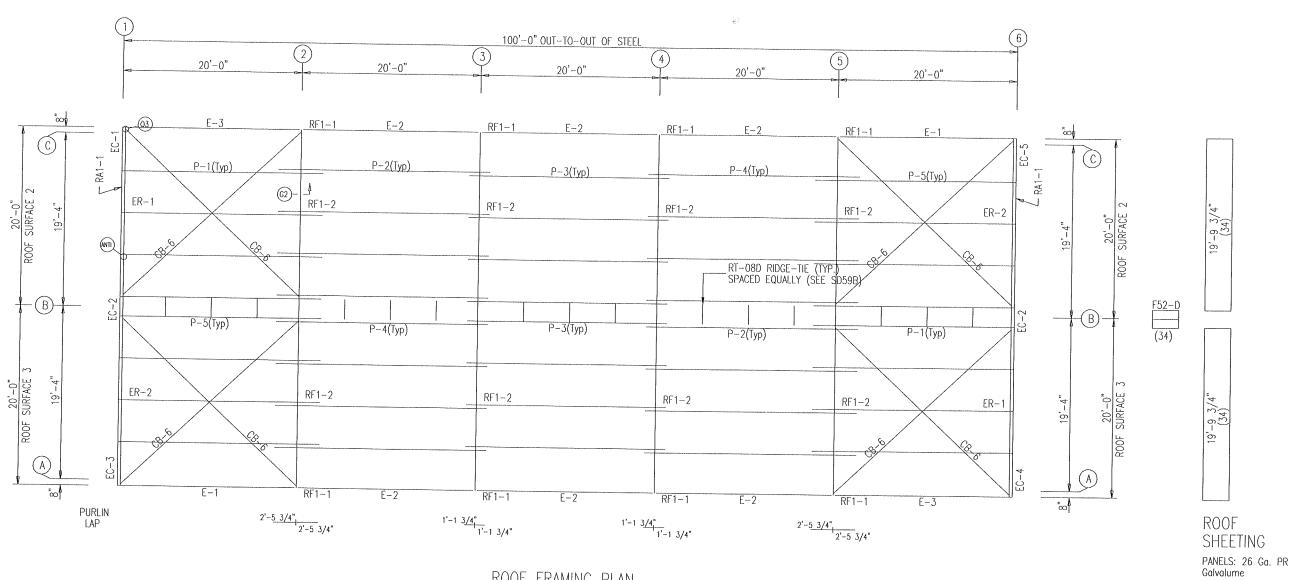
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	5/ 5/14	FOR ERECTOR INSTALLATION	GJO	SN	SAF			TURC) SIEE	r RAILL	DINGS INC			NO SOFE
									1405 [ENISON STF	REET			ANK ANK
									MARKH	AM, ON L3R-	-5V2			
						PROJECT:	PROJECT: GANESH VENAKATARAMAN T14-0087					HE WE	C 646	
						CUSTOMER:	GANESH VENAKA	TARAMAN		OWNER: GANESH VENAKATARAMAN				EXP. 6/30
						LOCATION:	OCATION: SAN MARTIN, CA 95046						-H	May 08, 2
_						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE	CIVI
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ROOF FRAMING PLAN

GENERAL NOTES:

- 1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
 2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
 3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
 4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
 5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEFTS AS NEFDED. SHEETS AS NEEDED.
- 6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

				т						
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN		TODO CTEEL DIMIDINGS IN			
A	5/ 5/14	FOR CONSTRUCTION PERMIT	GJ0	SN	SAF		TORO STEEL BUILDINGS INC			
							1405 DENIS	ON STREET		
							MARKHAM, ON L3R-5V2			
						PROJECT:	GANESH VENAKATARAMAN T14-0087			
	***************************************					T NOULCT.	GANESH VENAKATARAMAN 114-0087			
						CUSTOMER:	GANESH VENAKATARAMAN	OWNER: GANESH VENAKATARAMAN		
			ĺ			LOCATION:	SAN MARTIN, CA 95046			

DATE

5/ 5/14

SCALE

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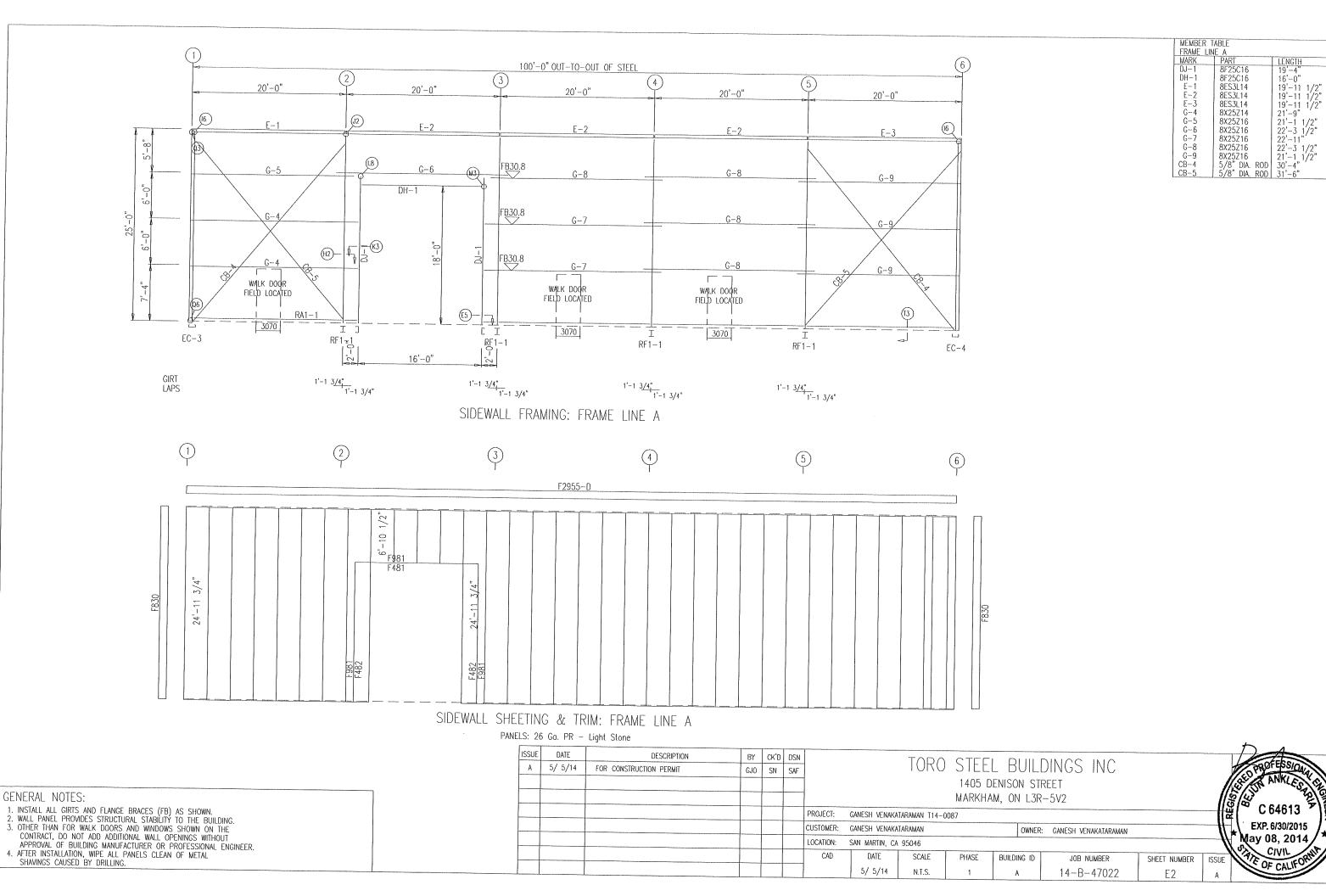
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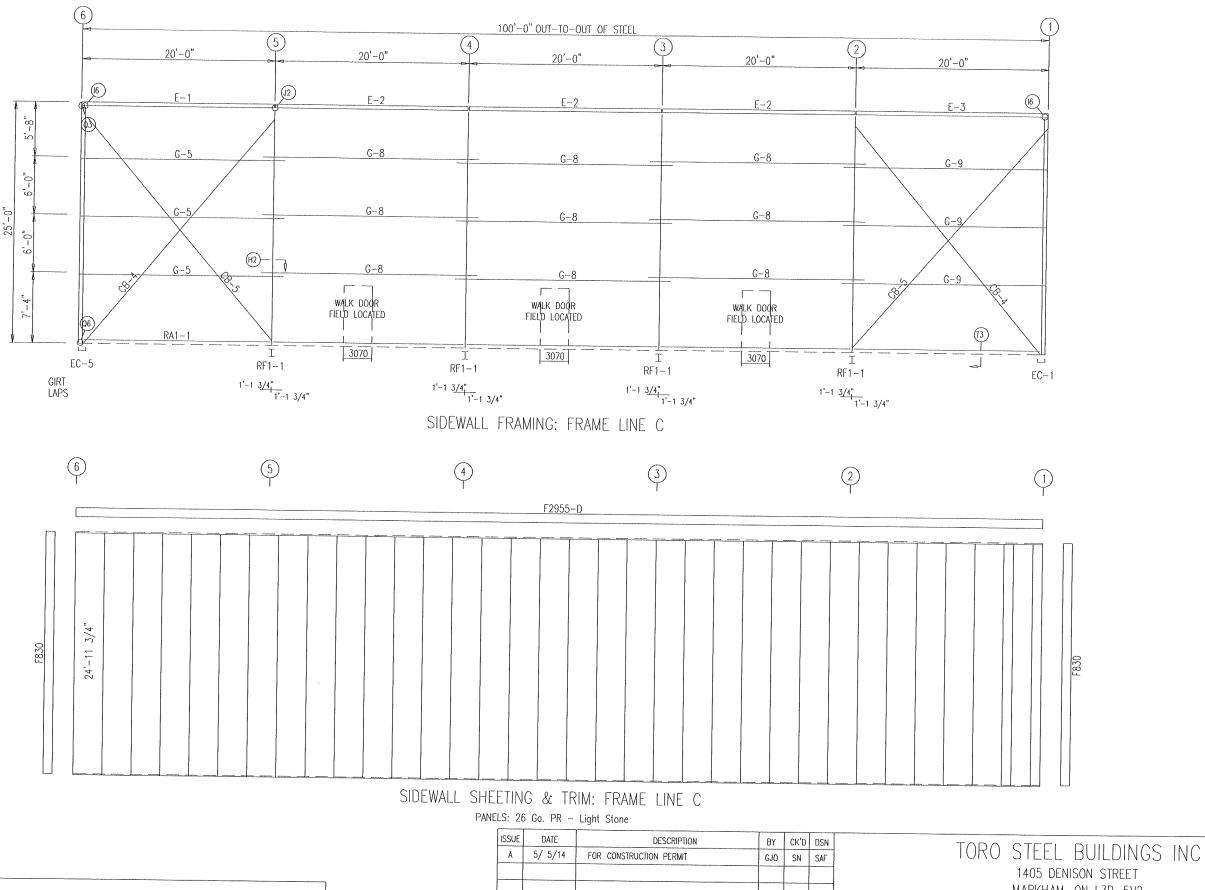
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14-B-47022

SHEET NUMBER

CAD





GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

MARKHAM, ON L3R-5V2

GANESH VENAKATARAMAN T14-0087

CUSTOMER: GANESH VENAKATARAMAN

LOCATION: SAN MARTIN, CA 95046

5/ 5/14

DATE SCALE PHASE

N.T.S.

BUILDING ID JOB NUMBER

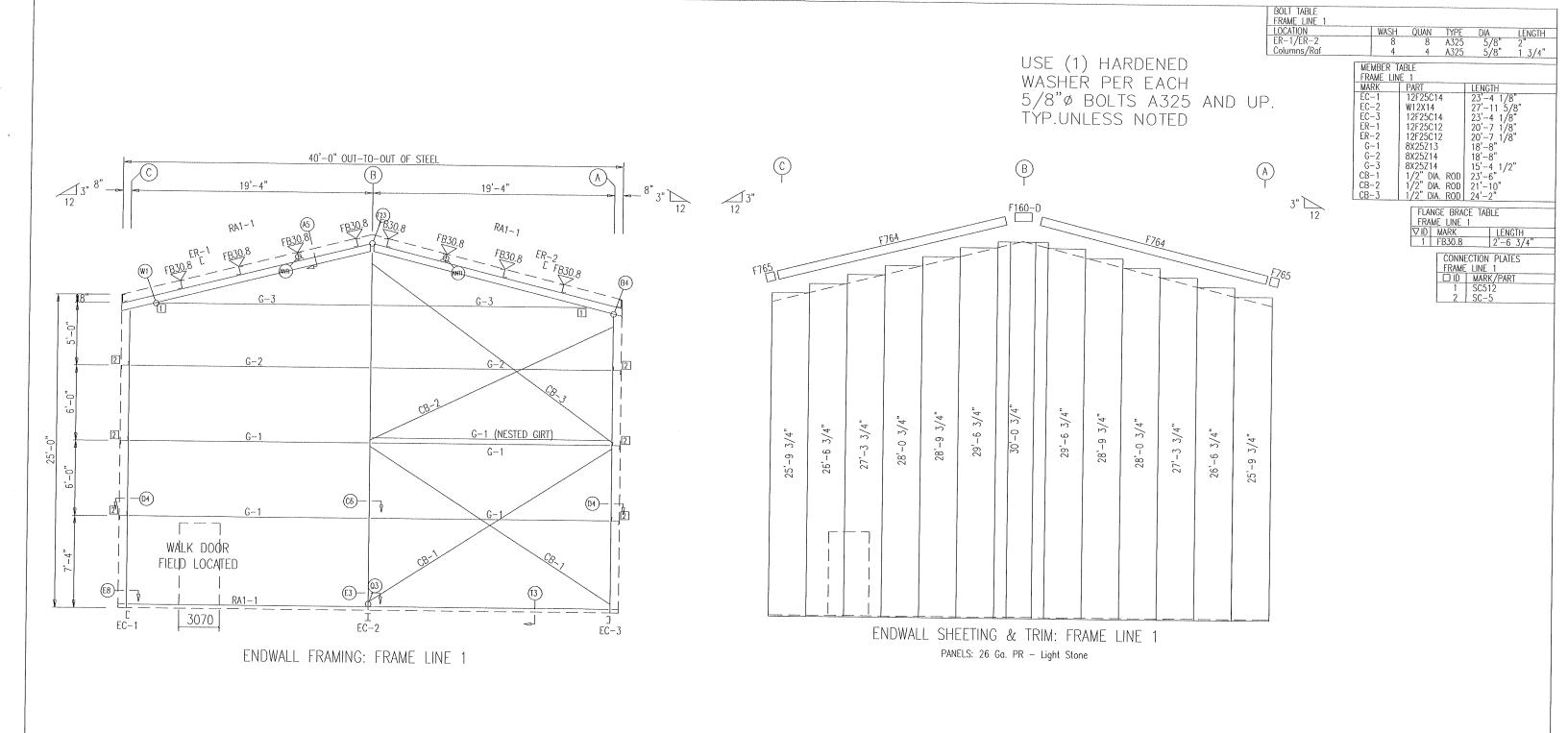
14-B-47022

C 64613 EXP. 6/30/2015 OWNER: GANESH VENAKATARAMAN May 08, 2014

MEMBER TABLE FRAME LINE C
MARK PAR

G-5 G-8 G-9 CB-4 CB-5

SHEET NUMBER E3



GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL STABLACE CALLEED BY DOUBLING.

SHAVINGS CAUSED BY DRILLING.

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<u> </u>		

DATE	DESCRIPTION	BY	CK,D	DSN	N TODO OTECH DI
5/ 5/14	FOR CONSTRUCTION PERMIT	GJO	SN	SAF	FI TORO STEEL BU
					1405 DENISON
					MARKHAM, ON
					PROJECT: GANESH VENAKATARAMAN T14-NN87

LOCATION:

CUSTOMER: GANESH VENAKATARAMAN

SAN MARTIN, CA 95046 DATE

5/ 5/14

SCALE

N.T.S.

PHASE

UILDINGS INC N STREET

L3R-5V2

BUILDING ID

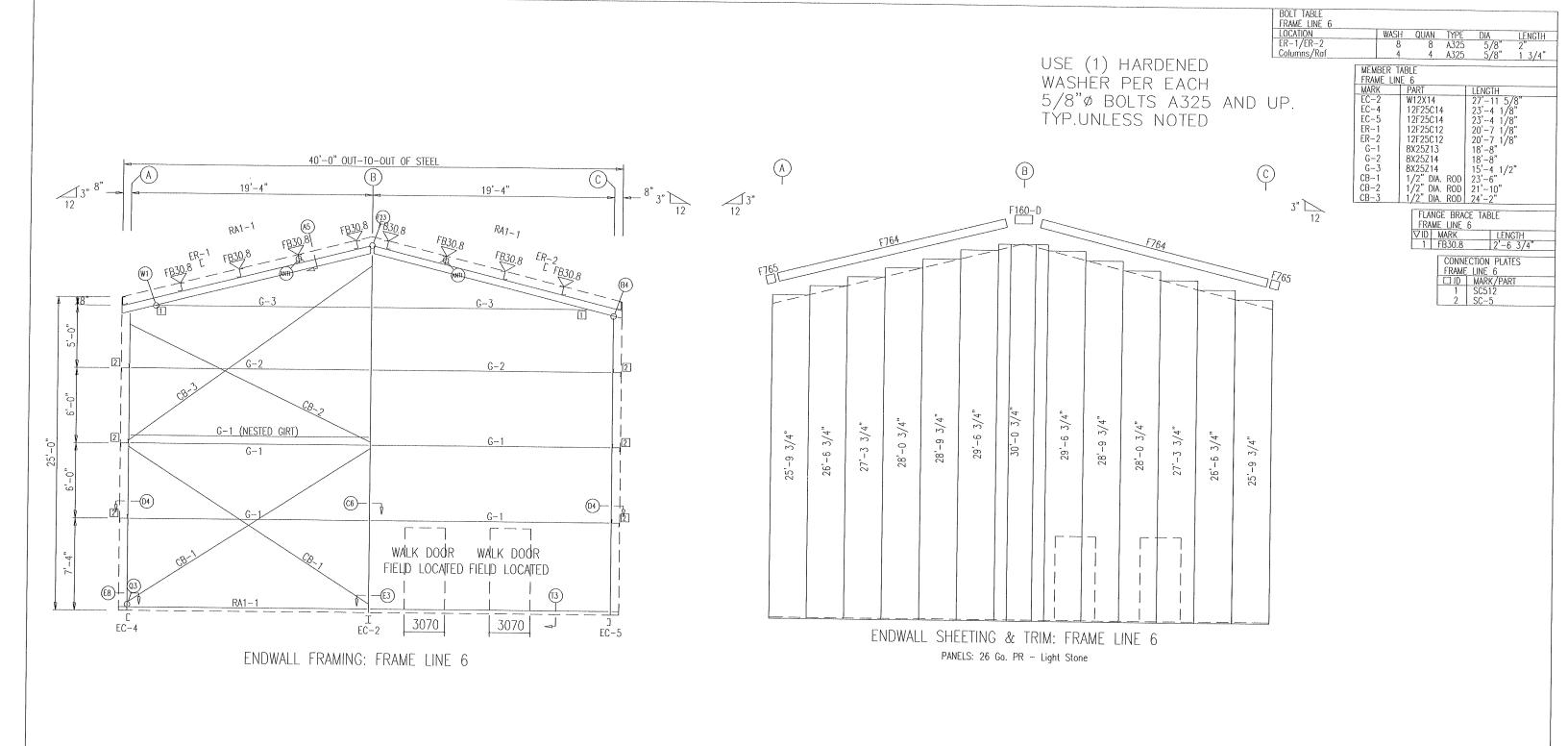
OWNER: GANESH VENAKATARAMAN

14-B-47022

EXP. 6/30/2015 **V**ay 08, 2014 JOB NUMBER SHEET NUMBER

E4

C 64613



GENERAL NOTES:

INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
 WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
 OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.

APTER MAGNALATION WHEN ALL DAMILES CLEAN OF METAL

4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

DATE	DESCRIPTION	BY	CK,D	DSN		TODO		2111 012100 1210
5/ 5/14	FOR CONSTRUCTION PERMIT	GJ0	SN	SAF		TURU S	oleel e	BUILDINGS INC
							1405 DENIS	ON STREET
						٨	MARKHAM, O	N L3R-5V2
					PROJECT:	GANESH VENAKATARAMAN T14-0087		
					CLISTOMER:	CAMECH VENAVATADAMANI	***************************************	OWNED OTHERWISE

SAN MARTIN, CA 95046

SCALE

N.T.S.

DATE

5/ 5/14

LOCATION:

OWNER: GANESH VENAKATARAMAN

PHASE BUILDING ID JOB NUMBER SHEET NUMBER

14-B-47022

C 64613 EXP. 6/30/2015 **V**ay 08, 2014

SPLICE BOLT TABLE Qty Qty Wash Top Bot Int Type Dia 4 0 A325 3/4"

USE (1) HARDENED

FLANGE BRACES: BOTH SIDES(UNLESS NOTED)
FBxxA(1): xx=length(in)
A - L2X2X14G

 Web
 Plate

 Thick
 Length

 0.134
 240.0

 0.134
 39.4

 0.156
 18.1

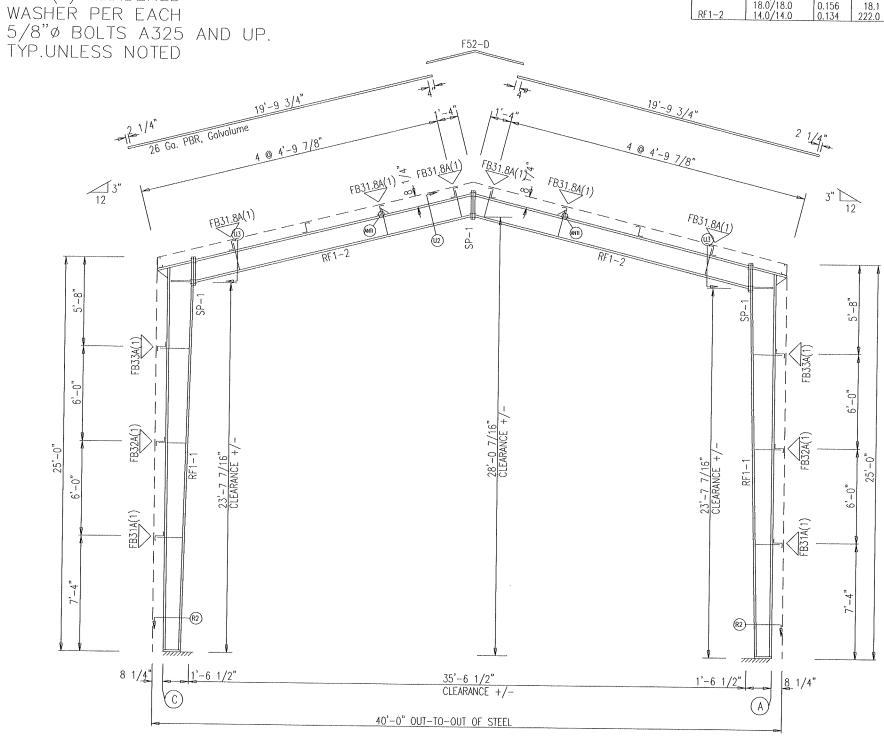
 0.134
 222.0
 Inside Flange W x Thk x Length

5 x 1/4" x 240.0

5 x 1/4" x 52.9

5 x 1/4" x 27.3

5 x 1/4" x 218.5 W x Thk x Length
5 x 1/4" x 240.0
5 x 1/4" x 39.5 RF1-1 5 x 1/4" x 218.5



RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5

GENERAL NOTES:

- 1. ALL BOLTED JOINTS WITH A325M-09 TYPE 1 BOLTS GREATER THAN 1/2" DIAMETER ARE SPECIFIED AS PRETENSIONED JOINTS IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, JUNE 30, 2004". PRETENSIONING CAN BE ACCOMPLISHED BY USING THE TURN-0F-NUT METHOD OF TIGHTENING, CALIBRATED WRENCH, TWIST OFF TYPE TENSION CONTROL BOLTS OR DIRECT TENSION INDICATOR AS ACCEPTABLE TO THE INSPECTING AGENCY AND BUILDING OFFICIAL. INSTALLATION INSPECTION REQUIREMENTS FOR PRE-TENSIONED JOINTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.2) USING TURN-0F-NUT METHOD IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SUP CLITICAL IS SUGGESTED. THE CONNECTIONS ON THIS PROJECT ARE NOT SLIP CLITICAL.
- 2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- 3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN

E	DATE	DESCRIPTION	BY	CK'D	DSN	TODO OTECH DIMENSO MA
	5/ 5/14	FOR CONSTRUCTION PERMIT	GJO	SN	SAF	TORO STEEL BUILDINGS INC
						1405 DENISON STREET
_						MARKHAM, ON L3R-5V2
		:				PROJECT: GANESH VENAKATARAMAN T14-0087

LOCATION:

CAD

CUSTOMER: GANESH VENAKATARAMAN

SAN MARTIN, CA 95046

DATE

5/ 5/14

PHASE

SCALE

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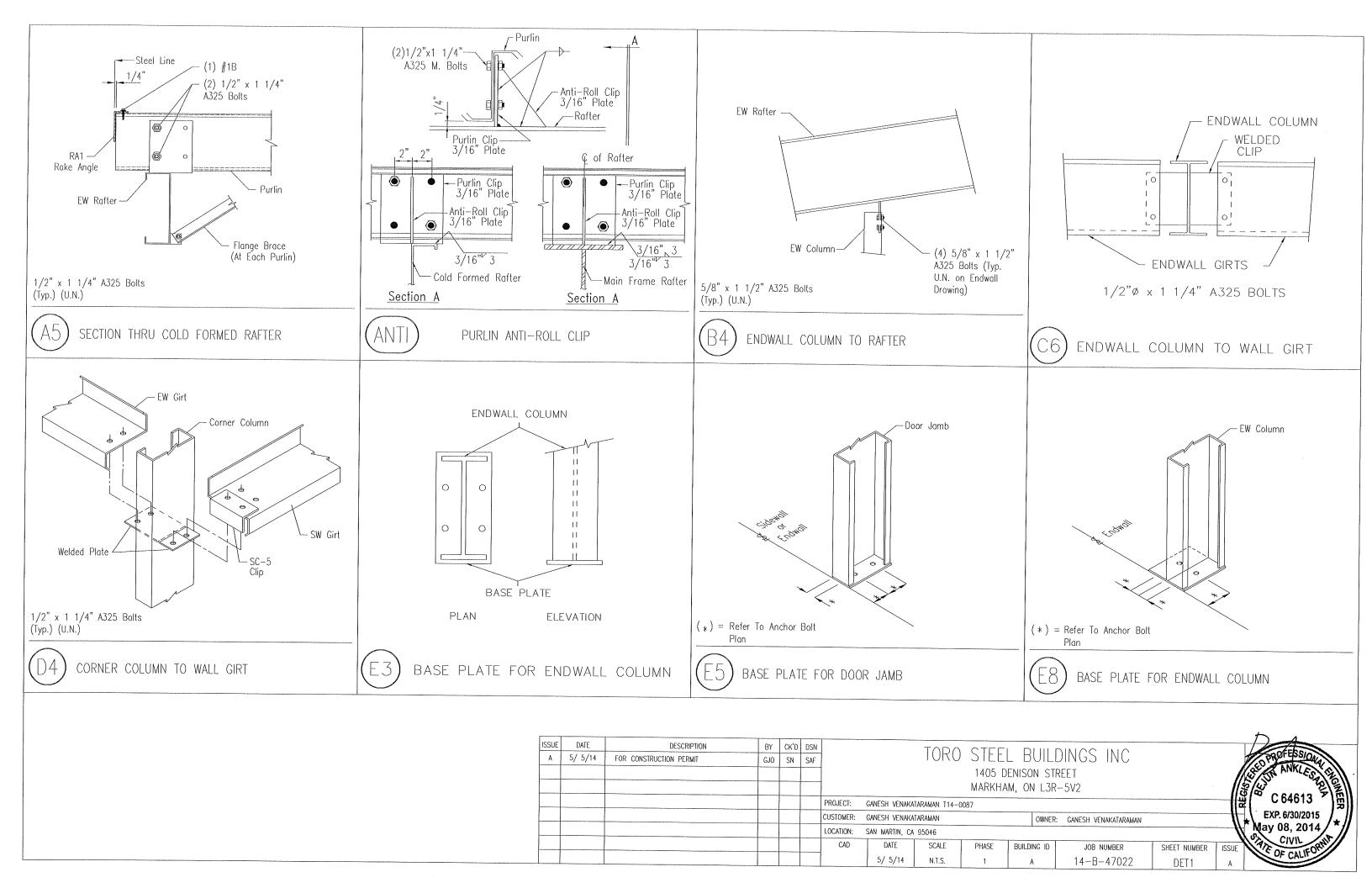
OWNER: GANESH VENAKATARAMAN

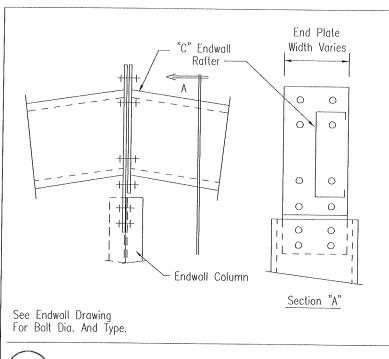
BUILDING ID JOB NUMBER SHEET NUMBER

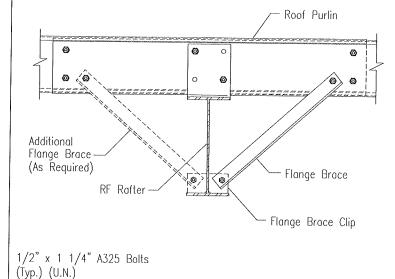
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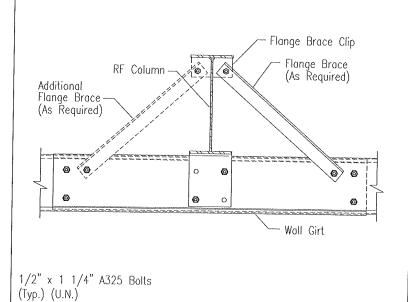
E6

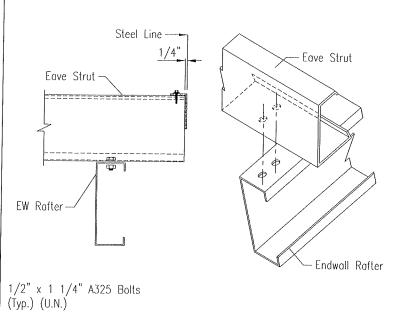










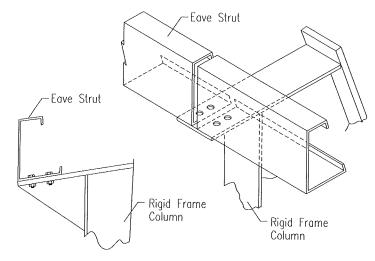


RAFTER SPLICE AT SURFACE CHANGE

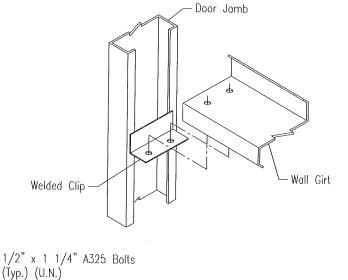
ROOF PURLIN TO INTERIOR FRAME RAFTER

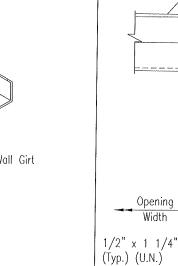
WALL GIRT TO RIGID FRAME COLUMN

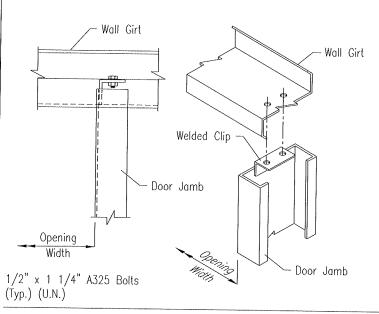
LOW SIDE EAVE STRUT TO COLD FORMED RAFTER

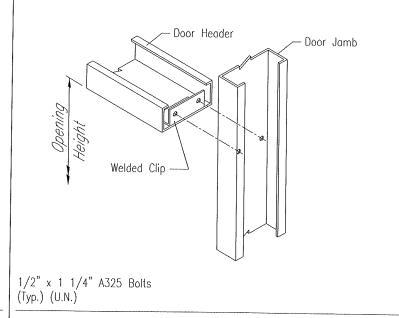












WALL GIRT TO DOOR JAMB

DOOR JAMB TO WALL GIRT

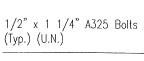
DOOR HEADER TO DOOR JAMB

14-B-47022

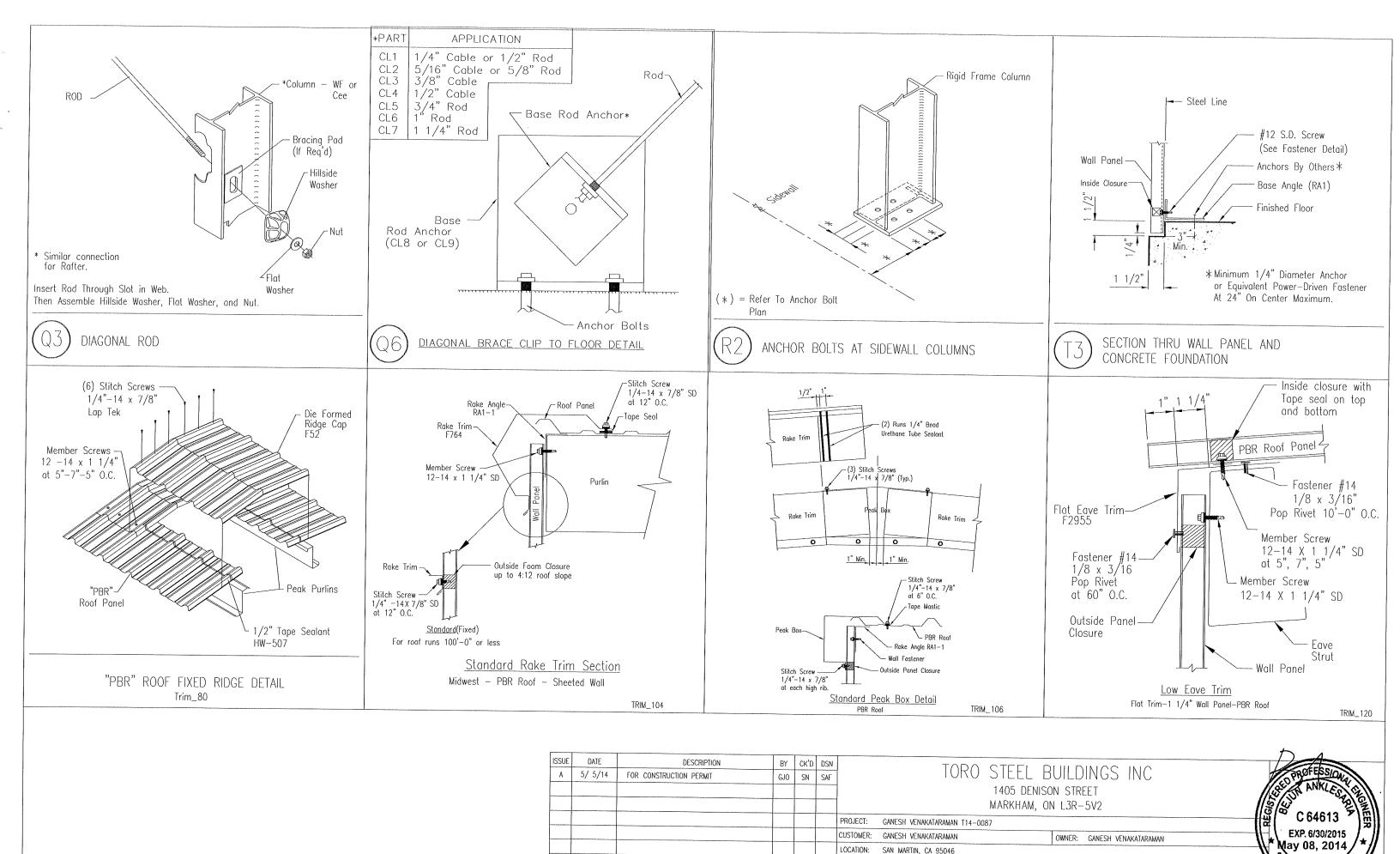
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									1405 D	DENISON S	STREET		1	//S	MI
						1			MARKHA	AM, ON L	.3R-5V2			\$ 18	<i>3</i> 3.
						PROJECT: (SANESH VENAKA	ATARAMAN T14-(0087				-40	#[]	C
	· · · · · · · · · · · · · · · · · · ·					CUSTOMER: C	GANESH VENAKA	ATARAMAN		1WO	NER: GANESH VENAKATARAMAN		-H		EXP.
						LOCATION: S	SAN MARTIN, CA	95046						7 °	lay 0
						CAD	DATE	SCALE	PHASE	BUILDING II	ID JOB NUMBER	SHEET NUMBER	ISSUF	1	à T

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5/ 5/14



EAVE STRUT TO RIGID FRAME



CAD

DATE

5/ 5/14

SCALE

N.T.S.

PHASE

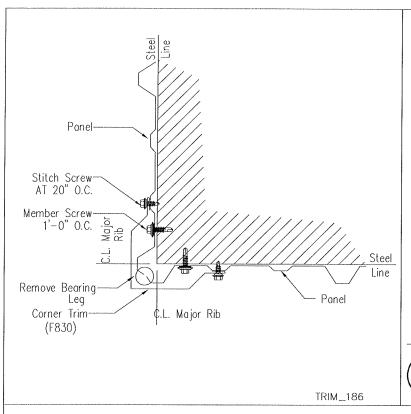
BUILDING ID

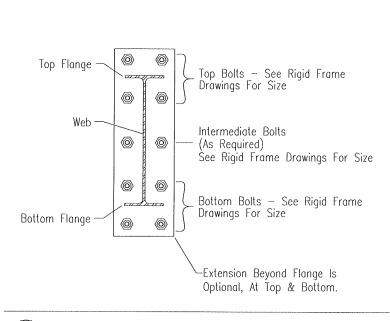
JOB NUMBER

14-B-47022

SHEET NUMBER

DET3

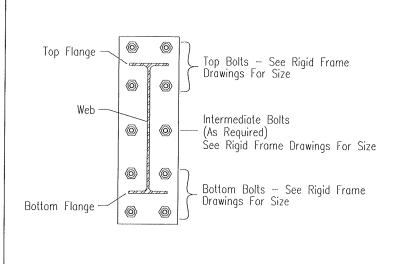


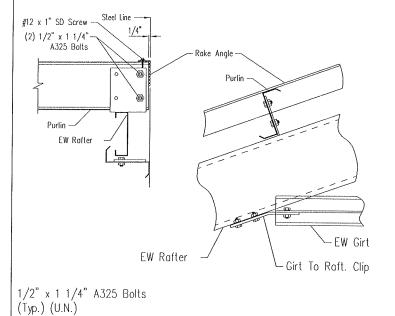


BOLTS FOR RIGID FRAME RAFTER

ISSUE

AT BUILDING PEAK





BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION

ENDWALL GIRT TO ENDWALL RAFTER CONNECTION

SHEET NUMBER

DET4

Standard Grade

Description	Fastener Number	Application
1/4"-14 x 7/8"	4A	Stitch & Trim Screw
12-14 x 1 1/4"	17A	Member Screw
12-14 x 1 1/2"	17B	Member Screw
12-14 x 2"	28	Member Screw

Long Life

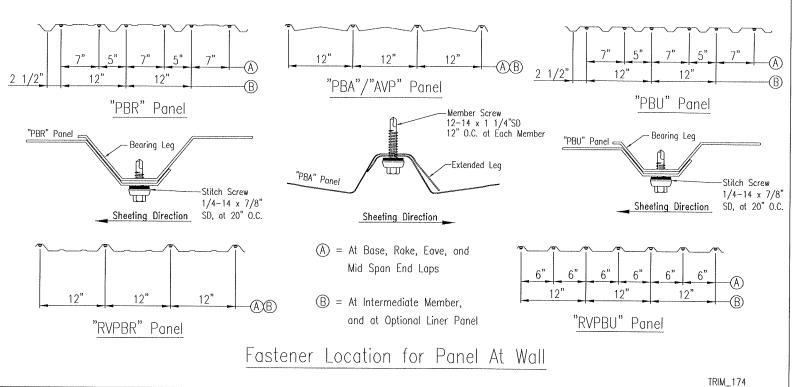
Description	Fastener Number	Application
1/4"-14 x 7/8"	4	Stitch & Trim Screw
12-14 x 1 1/4"	3	Member Screw
12-14 x 1 1/2"	3A	Member Screw
12-14 x 2"	58	Member Screw

Note:

Standard details call for 1 1/4" fasteners as member screws by default.

Member screws may be 1 1/4", 1 1/2", or 2" depending on insulation, application, or customer request.

<u>Self-Drilling</u> Screw Application



·					
DATE	DESCRIPTION	BY	CK,D	DSN	TODO CTEEL DUMDINGS INC
5/ 5/14	FOR CONSTRUCTION PERMIT	GJO	SN	SAF	TORO STEEL BUILDINGS INC
					1405 DENISON STREET
					MARKHAM, ON L3R-5V2
					PROJECT: GANESH VENAKATARAMAN T14-0087
					CUSTOMER: GANESH VENAKATARAMAN OWNER: CANESH VENAKATARAMAN

SAN MARTIN, CA 95046 DATE

5/ 5/14

SCALE

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PHASE

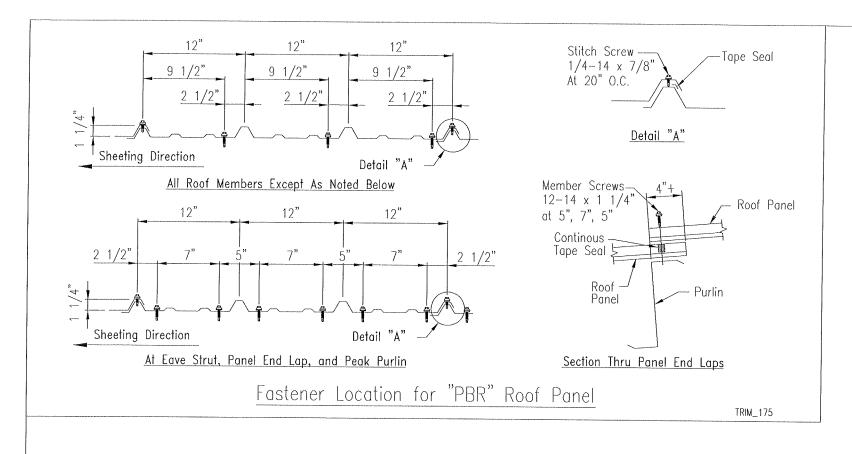
BUILDING ID

JOB NUMBER

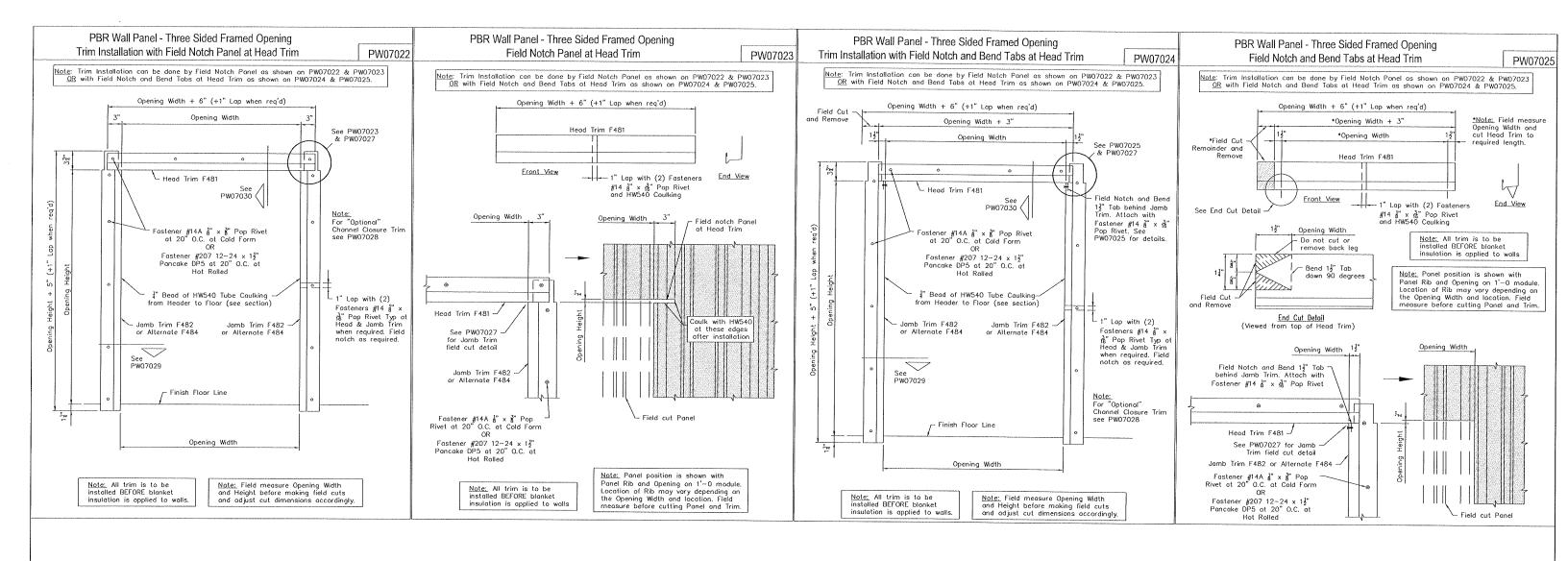
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LOCATION:

EXP. 6/30/2015 **V**ay 08, 2014



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										AM, ON L3F	(-3/2		//š	C 64613
						PROJECT:	GANESH VENAKA	TARAMAN T14-	0087				#	
						CUSTOMER:	GANESH VENAKA	TARAMAN		OWNER	: GANESH VENAKATARAMAN		-11	EXP. 6/30/2015
						LOCATION:	SAN MARTIN, CA	95046					-H	May 08, 2014
						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE	CIVIL ORNIE
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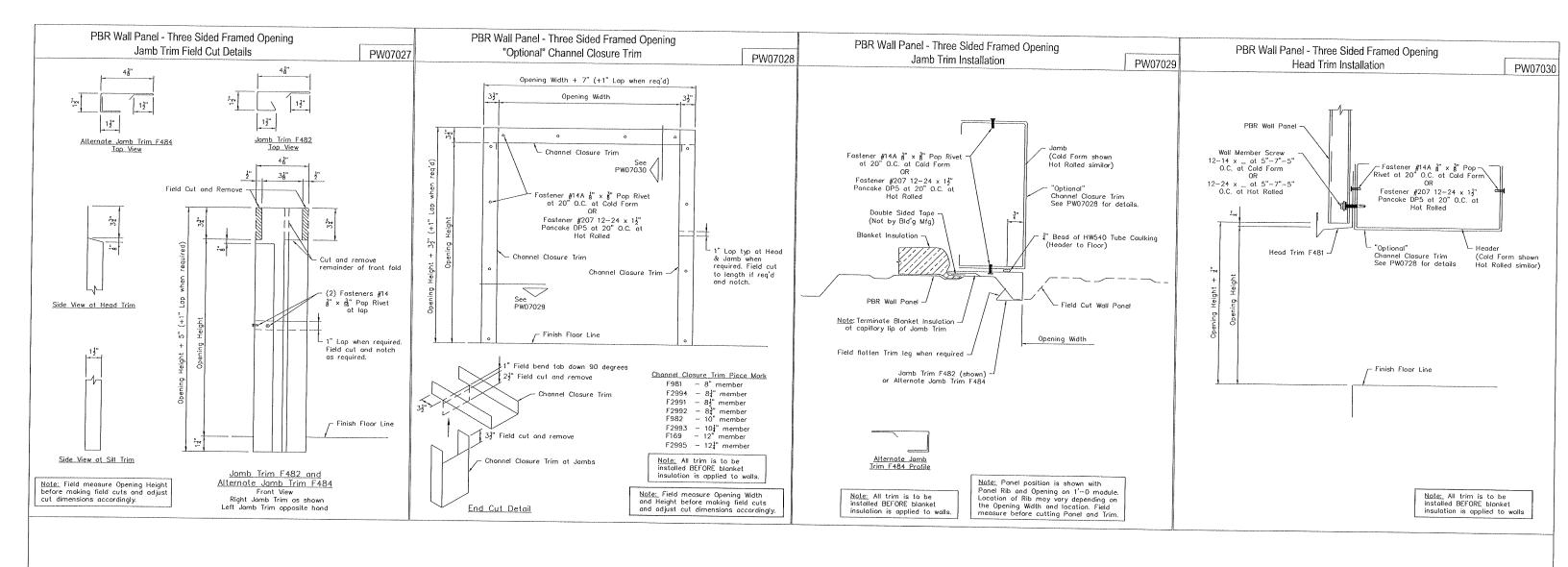


STANDARD FRAMED OPENING DETAILS (PBR WALL PANEL)

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									1405 E	ENISON ST	REET		
									MARKHA	AM, ON L3R	-5V2		
						PROJECT:	GANESH VENAKAT	ARAMAN T14-	0087			***************************************	
						CUSTOMER:	GANESH VENAKAT	TARAMAN		OWNER	: GANESH VENAKATARAMAN		_#
						LOCATION:	SAN MARTIN, CA	95046		1, , , , , , , , , , , , , , , , , , ,			
						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
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EXP. 6/30/2015 May 08, 2014



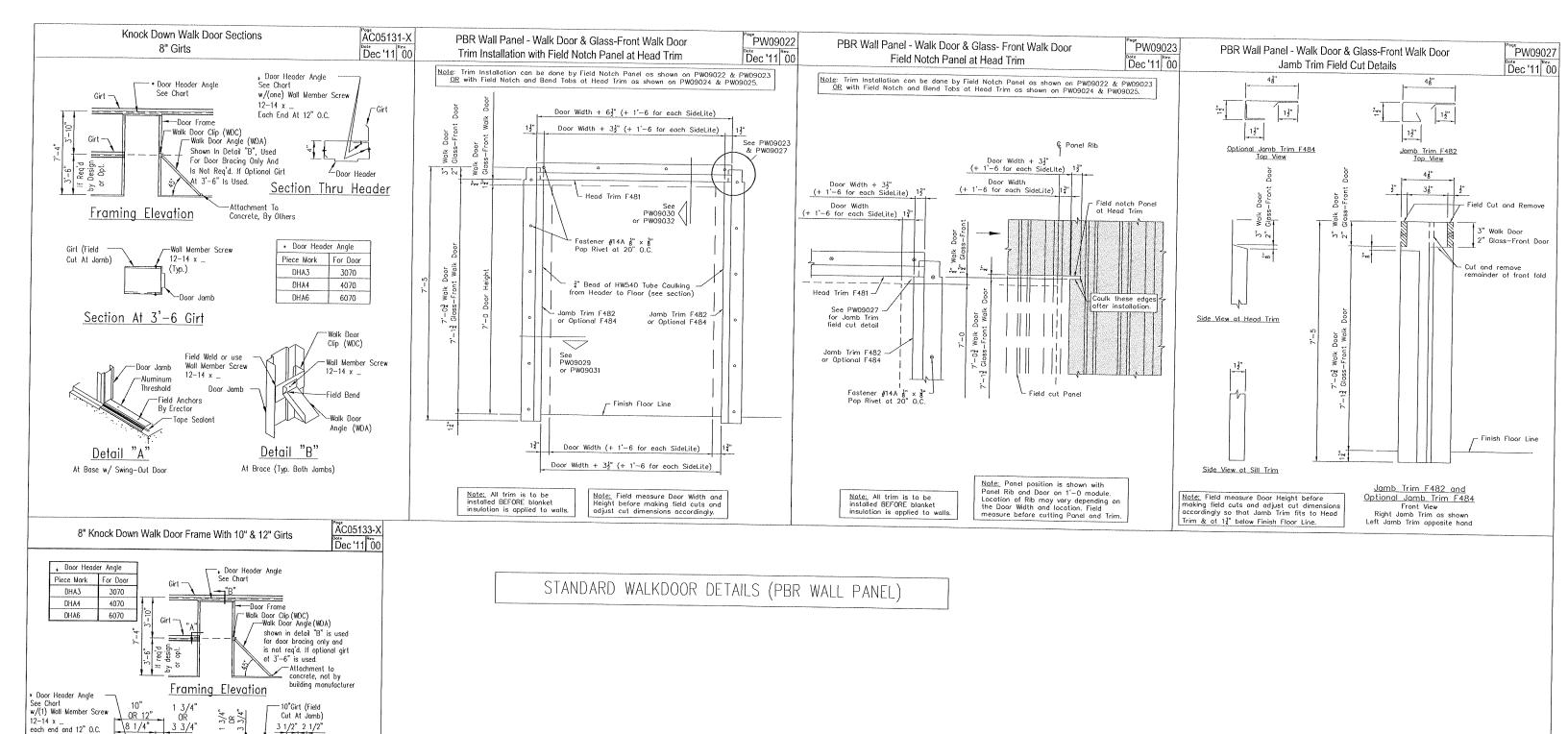
STANDARD FRAMED OPENING DETAILS (PBR WALL PANEL)
CONT.

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									1405 D	ENISON ST	REET		
	****								MARKHA	AM, ON L3F	R-5V2		
						PROJECT:	GANESH VENAKA	TARAMAN T14-	0087				-H
						CUSTOMER:	GANESH VENAKA	TARAMAN		OWNER	R: GANESH VENAKATARAMAN		
						LOCATION:	SAN MARTIN, CA	95046	····				
						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
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C 64613

EXP. 6/30/2015 May 08, 2014

OF CALIFOR



-(3) Wall Member Screw 12-14 x _

----Door Frame

- Walk Door Clip (WDC)

✓ Wall Member Screw

12-14 x

-Field Bend

(ACM)

ploin material angle RA1-_ x 0'-6"

Section "A" At 3'-6" Girt

Detail "B"

At Brace (Typ. Both Jambs)

Door Frame

-Door Jamb

-Aluminum Threshold

Field Anchors By Erector

Wall Member Screw

Door Jamb

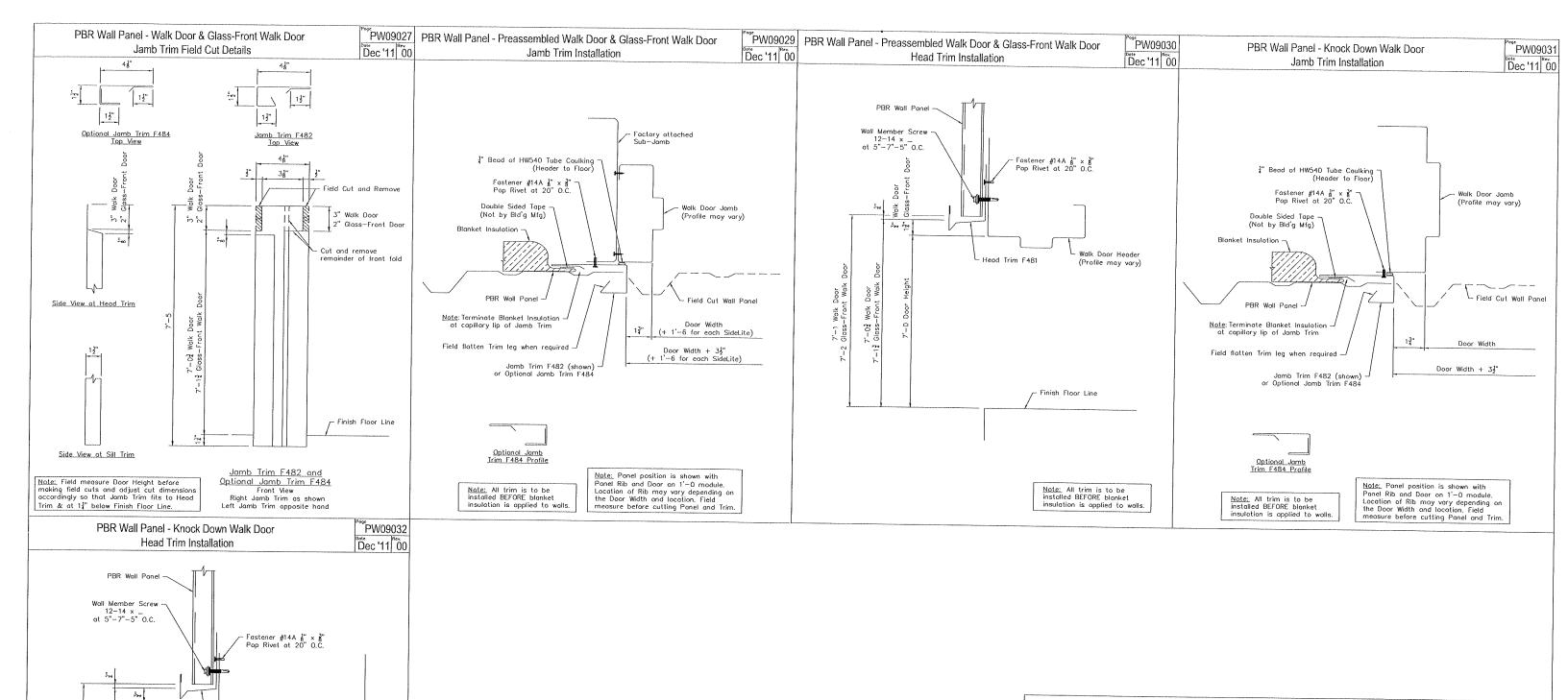
12-14 x _

Section "B" Thru Header

<u>Detail "A"</u>

At Base w/ Swing-Out Door

ISSUE	DATE	DESCRIPTION	BY	CK,D	DSN			TADA	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	-1 0111				DA
A	5/ 5/14	FOR CONSTRUCTION PERMIT	GJO	SN	SAF			TURC) SIEE	T BAI	LDINGS INC			PROFESSION.
										DENISON : AM, ON L				ANKLEGE CH
						PROJECT:	GANESH VENAKA		-0087				- \u00e4	C 64613
						CUSTOMER:	GANESH VENAKA	ATARAMAN	***************************************	OWN	ER: GANESH VENAKATARAMAN		-H	EXP. 6/30/2015
						LOCATION:	SAN MARTIN, CA	95046						May 08, 2014/ */
						CAD	DATE	SCALE	PHASE	BUILDING II	JOB NUMBER	SHEET NUMBER	ISSUE	CIVIL
<u></u>							5/ 5/14	N.T.S.	1	A	14-B-47022	DET8	A	OF CALIFO



- Head Trim F481

7.-03

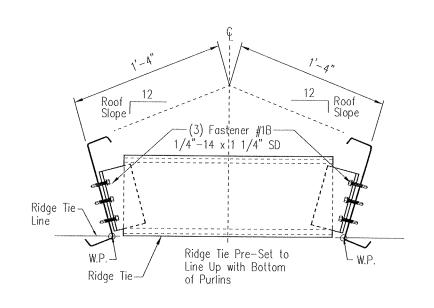
Walk Door Header (Profile may vary)

- Finish Floor Line

Note: All trim is to be installed BEFORE blanket insulation is applied to walls.

STANDARD WALKDOOR DETAILS (PBR WALL PANEL)
CONT.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN			TADA		-1 0			****		$\top/$	2
A	5/ 5/14	FOR CONSTRUCTION PERMIT	GJO	SN	SAF	1		TURC) SIEE	:L Bl	JILU	INGS INC			1	9
										DENISON						(5
							-		MARKH	AM, UN	L3R-	-5V2			3/8	7
						PROJECT:	GANESH VENAKA	TARAMAN T14-	0087						¥ ((
						CUSTOMER:	GANESH VENAKA	TARAMAN			OWNER:	GANESH VENAKATARAMAN		-H	10 Y	E)
						LOCATION:	SAN MARTIN, CA	95046						$-\mathcal{H}$	(M	ay
						CAD	DATE	SCALE	PHASE	BUILDIN	G ID	JOB NUMBER	SHEET NUMBER	ISSUE		ァ
							5/ 5/14	N.T.S.	1	A		14-B-47022	DFT9	Δ .	1	



Roof slope	8" Purlin	10" Purlin	12" Purlin
1/2	RT08 A	RT10 A	RT12 A
1	RT08 B	RT10 B	RT12 B
2	RT08 C	RT10 C	RT12 C
3	RT08 D	RT10 D	RT12 D
4	RT08 E	RT10 E	RT12 E

Ridge Tie Installation

NOTE: These drawings are intended to depict general installation of item(s) described above. Some item(s) may have been omitted for clarity of presentation. Consult your erection manual or additional SD—Sheets for further guidelines and/or clarifications.

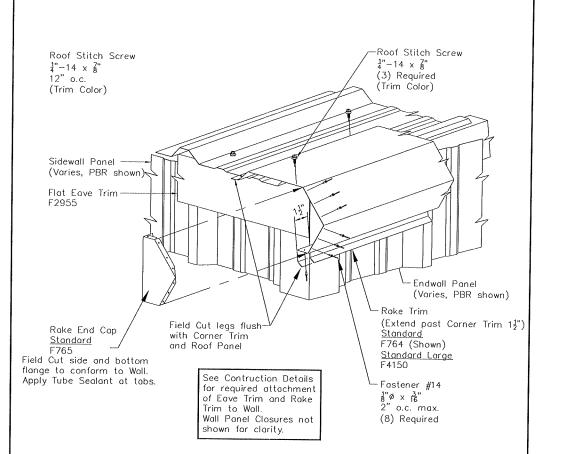
Created On: 08/21/09

DRAWNG NO.

SD59B

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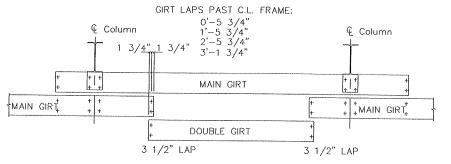
PBR Roof Panel - Standard and Standard Large
Low Eave Rake Corner with Flat Eave Trim - 1 4 Wall Panel



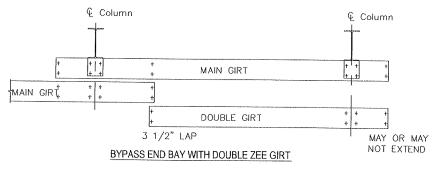
Double Member Zee-Section Welded Girt

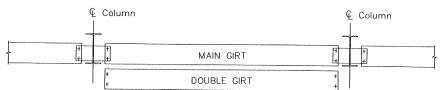
21-05-26 Dec '12 00

DOUBLE MEMBER ZEE—SECTION GIRT WILL RUN TO THE END OF THE MAIN GIRT IT'S NESTED WITH UNLESS ANOTHER MAIN GIRT LAPS INTO THE BAY. IN THAT CONDITION THE DOUBLE GIRT WILL STOP AND BOLT TO THE END OF THE LAPPING MAIN GIRT, AS SHOWN BELOW. THE STANDARD LAP OF THE NESTED DOUBLE GIRT TO MAIN GIRTS IS 3 1/2".



BYPASS INTERIOR BAY WITH DOUBLE ZEE GIRT





FLUSH AND INSET BAY WITH DOUBLE ZEE GIRT

JOB NUMBER

14-B-47022

IF FLANGE BRACE EXTENDS BEYOND LAP REQUIREMENTS, THE NESTED GIRT WILL NEED TO BE PUNCHED AS WELL.

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DATE	DESCRIPTION	BY	CK'D	DSN	TODO CTEEL DUILDINGO ING	•
/ 5/14	FOR CONSTRUCTION PERMIT		SN	SAF	TORO STEEL BUILDINGS INC	
					1405 DENISON STREET	
				MARKHAM, ON L3R-5V2		
					PROJECT: GANESH VENAKATARAMAN T14-0087	
					CUSTOMER: GANESH VENAKATARAMAN OWNER: GANESH VENAKATARAMAN	

SAN MARTIN, CA 95046

SCALE

N.T.S.

PHASE

BUILDING ID

DATE

5/5/14

LOCATION:

X-WPR04006

C 64613

EXP. 6/30/2015

May 08, 2014

CIVIL

OF CALIFORNIA

SHEET NUMBER

DET10

